

Review Paper

Seismic Presentation of a Multi-Story Building Incorporated with Different Base Isolation Devices Using Pushover Analysis

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Handling Editor: Sparsh Sharma

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Pushover analysis was started used widely in the year 1970 but it's capability and excellent performance was identified in last 20 years. This technique is majorly used to predict the potency and glide capacity of already existing structure and shaking requisite for the same existing structure which are subjected to extremely strong earthquakes. This technique can also be employed for verifying capability newly presented structural design. Because of pushover analysis being effective it brought changes to various guidelines like ATC-40 and FEMA-356 and also some changes to design codes like Euro Code 8 and PCM 3724 in past some years. Pushover analysis is defined as an analysis in which a mathematical representation openly including the nonlinear weight bending distinctiveness of personal mechanism and fundamentals of the structure shall be subjected to monotonically rising imaginative loads representing inactivity services in a shaking until a objective dislocation is going beyond. Maximum displacement of the structure is the target displacement which is at the roof and is predictable under various selected ground motion of earthquake.

1. Introduction

In last 20-30 years structure made by using steel has been playing a very significant position in the commerce of construction. It

is essential to mean a structure to execute well underneath seismic loads. If shear capacity of a structure is to be increased it can be done by introduction of steel bracings in structural arrangement of the

building. Other than for increasing shear capacity bracings can be considered retrofit as well. 'N' number of possibilities are available to position the steel bracings. Some of the available bracings include D type, K type and V type eccentric invigorating [1]. Such type of structures should always include good ductility properties so that their performance should be well under any type of seismic loads. If estimation of ductility and other important properties is to be done that can be only done by performing Push over Analysis [2,3]. A uncomplicated computer-based pushover examination is a procedure for routine based plan of structure frameworks focused to seismic activity weight. In past years pushover analysis has accomplished majority of importance because of it being simple to perform and giving effective results. The present study extends the pushover analysis on steel frames with various base isolations. Various base isolation devices used in this study are high damping rubber bearing, low damping rubber bearing and lead rubber bearing [4,5].

Steel is without a doubt nearly only practical material for construction of building globally. In today's world industry of steel is key business in any country. Its rough calculation potency is 10 times more than concrete and is perfect material for

construction in today's world. Major advantages of steel are its strength, its movement of erection, preproduction and demountability. Structural steel is second-handed in handling of loads in frames of the building and also as constituent in trusses of bridges and frames of spaces [6]. Steel is very useful in every type of construction but its requirement to be comfortable for usage is that it should be free from corrosion and also protection from fire. Buildings made from steel are provide with claddings and dividing walls which are ceated up of materials or masonry and frequently foundation of concrete is offered. Steel is also incorporated in coincidence frame and construction of shear wall. Because of its great strength to weight relation structures made from steel are more economical in comparison to structure made from concrete usually for high rise buildings and high width buildings and also in various bridges. Construction by using steel is usually completed in less time and thus structure can be used early [7]. Steel presents better compressive strength and better tensile strength as compared to concrete and therefore facilitates lighter construction. If many benifits are to be availed from steel, structure made from steel should be made protective from fire and also resistant to corrosion. It should be usually intended and comprehensive for easy invention and manufacturing. Good

superiority management is necessary to make sure appropriate fitting of a variety of structural elements [8]. Effects usually by temperature should always be measured while constructing steel structures. Steel structures have capability to handle high seismic loads like earthquakes. Steel structures can also be repaired and retrofitted in order to carry very heavy loads [9]. Steel can also be considered environmentally friendly material and is very much 100% remanufacturable.

The base isolation procedure is a seismic intend move towards in which because of the inclusion of a stretchy coating sandwiched between the groundwork and the superstructure the basic occurrence of the arrangement reduces to a importance inferior than the major force including frequencies of tremor earth movement. Additionally, the capacity of damping which is offered usually by damping system helps in dispersing the power conveying while the occurrence of seismic activities [10, 11]. Seismic base machinery which is now known as the most important and resourceful machinery can be used for improvement of seismic presentation of most valuable building-like schools, hospitals, industries and high-rise buildings [12]. For reducing the drifts between inter storey and for minimizing the acceleration of floors the technique of base isolation is

mostly employed. Various base isolation devices to be used in this study include:

- High damping rubber bearing
- Low damping rubber bearing
- Lead rubber bearing

2. RELATED WORK

Modelling the structures in its whole is very complicated process to tackle. A short overview of preceding research of pushover analysis application is highlighted in this section. The review of literature focuses on discussion being done recently which is related to pushover analysis which meets the relation with the present study.

Wright and Humur (2017) had studied the behaviour of multi storeyed buildings dynamically. The conclusions of the study were as follows. The basic period was reduced by 37% for setback of 91%. The advanced form shaking of hinder buildings made considerable input to their seismic reaction, these offerings amplified with the thinness of the tower. Shahroz and Hawali (2016) had intentional the effects of impedeness on the shaking activity of different storeyed buildings. In this study an effort was put for improvement of design methods for impeding structures. For that reason, an investigational and systematic study was undertaken. Woods (2017) scrutinized the shaking presentation of concrete reinforced frames with different

steps and hinder. 2 different 12 storeyed concrete test enclosed structures were made and were subjected to ground motion. The dislocation, hastening and shear force responses of these frames was compared with formerly tested 8 different regular frames.

Ghobarah A (2016) had studied that the inter storey glide can also be measured as a resource to offer identical ductility over the stories of the structure. A storey shift might consequence in the incidence of a relatively weak storey which might be responsible for causing building collapse of catatrophic nature in a tectonic event. Identical anecdote ductility over different stories for a structure is regularly preferred in seismic propose. Foley CM (2017) had provided a appraisal of present position of the art seismic presentation based procedural designs and provided the idea for the expansion of optimization via PBD. I t is predictable that there is a imperative requisite for expansion of optimization via PBD methods for seismic structural engineering. B. Akbes (2018) had performed the pushover analysis on frames made by steel to evaluate the demands of seismic activity at dissimilar presentation levels which necessitate the deliberation of behavior of organization inelastically. R. Hasan and D.E. Greirson (2017) had performed a simple pushover analysis on

computer for evaluating or assessing the performance of frames of building subjected to heavy seismic activity loading. They found in their assessment that inflexibility reason for expandable investigation of partially unbending border and the properties of stiffness for partially inflexible investigation are straighty accepted for pushover analysis.

Ogus and Sermeno (2018) had determined the effects and exactness of non alaternative load laterally and consumed in using pushover analysis to estimate the behavior applied on structure because of erratically selecting personal ground motions responsible for cause flexible buckle by inmvestigating various stages of nonlinear answer. The perfectness of exact methods exploits to predict objective disarticulation was also studied on structure made of frames. The study concluded by saying that result obtained by pushover analysis were perfect and strong enough. Ahmed et al (2016) had stated in his study that due to it being simple pushover analysis technique is being used by structural engineers all over the world. I t is usually being used to carry out the nonlinear hinge properties which are available in various programs like FEMA-358 and ATC-45. The study also mentioned that length of plastic hinge has significant effects on the dislocation competence of the frames. Girgon (2015) had mentioned in its study that pushover analysis is the major used method for the assessment of seismic evaluation of structures by following various guidelines and also by different codes because it is simple both by concept and by computation. Pushover analysis can also be employed for performance of overall capacity curve of the preceding structure.

3. Results and Discussion

Modelling of 3 storey structure was done and pushover analysis on same was done using Staad Pro v8i software. After modeling table and graphs were plotted showing displacement with isolator in x-direction, displacement without isolator in y-direction and frequency with different base isolation devices and with fixed base and also details of bearing are mentioned in detail.

Modelling and Analysis of a 3 storey structure

Table 1 provides complete information on bearing. Phase of era assumed to be 1.9 seconds, Rigidity of High damping rubber bearing = 304 kN/m, Rigidity of Low damping rubber bearing 293 kN/m, Rigidity of rubber bearing 167 kN/m.

Table 1. Information on bearing on 3 storey structure

| No | Bearing Size | High dampin g bearing | Low dampin g bearing | Lead Rubbe r bearin g |
|----|-------------------|-----------------------|----------------------|-----------------------|
| i | Bearing dia | 200m m | 240m m | 300m m |
| ii | Width of personal | 5mm | 6mm | 7mm |

| | | | | |
|-------|---|--------|--------|--------|
| | rubber layer | | | |
| iii | Total layer of rubbers | 9 | 7 | 17 |
| iv | Total no of plates made of steel | 8 | 7 | 16 |
| v | Thicknes s of summit and undernea th plates made of steel | 11mm | 11mm | 11mm |
| vi | Breadth of personal plates made of steel | 1.2m | 1.2m | 1.2m |
| vii | Bearing tallness | 140m m | 120m m | 292m m |
| Vii i | Lead central part dia | - | - | 40mm |

Frequencies for these corresponding types are mentioned above which are usually taken from dynamic pushover analysis being done in Staad Pro v8i software which is also represented graphically and plot is

shown in 4 different colours and graphical plottation is done using Ms Excel software.

Table 2: Frequency of 3 storey structure including and excluding base isolation devices

| Type | Permanent bottom | Including High damping bearing | Including low damping bearing | Including lead rubber bearing |
|------|------------------|--------------------------------|-------------------------------|-------------------------------|
| i | 0.347 | 0.347 | 0.190 | 0.152 |
| ii | 0.358 | 0.358 | 0.285 | 0.261 |
| iii | 0.403 | 0.403 | 0.347 | 0.347 |
| iv | 0.500 | 0.500 | 0.470 | 0.468 |
| v | 0.636 | 0.636 | 0.535 | 0.521 |
| vi | 0.657 | 0.657 | 0.657 | 0.657 |

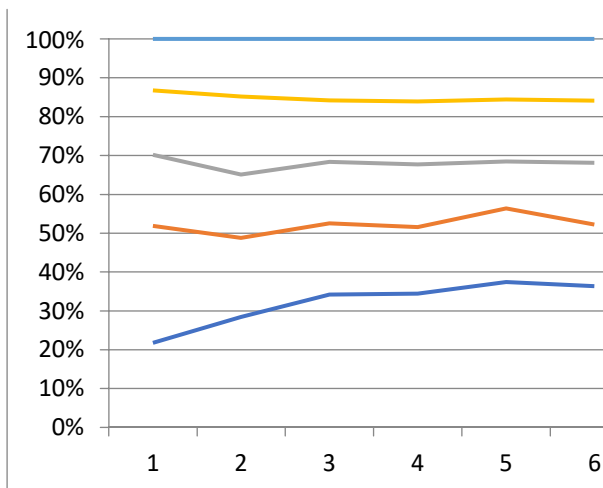


Figure 1: Graphical view of frequency-type of 3 storey structure

Table 3 and 4 below shows the displacements in both x and y direction for

3 storey structures including and excluding base isolation system.

Table 3: Displacement in x direction for 3 storey structure

| Storey number | Case of load | Dislocation excluding isolator (mm) | Dislocation including isolator (mm) |
|---------------|--------------|-------------------------------------|-------------------------------------|
| Ground | Push level x | 71 | 590 |
| 1 | Push level x | 160 | 592 |
| 2 | Push level x | 270 | 594 |
| 3 | Push level x | 340 | 596 |

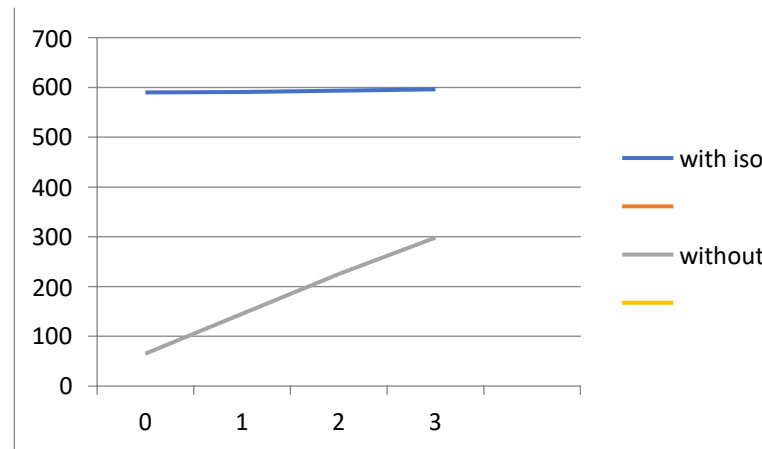


Figure 2: Dislocation in direction x graphically

Table 4: Displacement in y direction for 3 storey structure

| Storey number | Case of load | Dislocation excluding isolator (mm) | Dislocation including isolator (mm) |
|---------------|--------------|-------------------------------------|-------------------------------------|
| | | | |

| | | | |
|--------|--------------|-----|-----|
| Bottom | Push level y | 65 | 590 |
| 1 | Push level y | 145 | 591 |
| 2 | Push level y | 225 | 593 |
| 3 | Push level y | 298 | 596 |

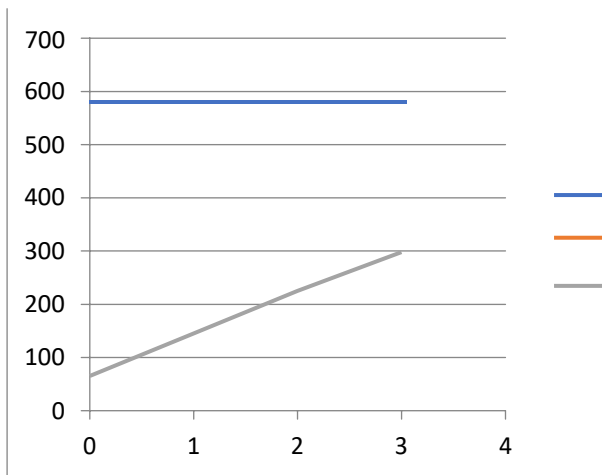


Figure 3: Displacement in Y direction for 3 storeyed structure graphically

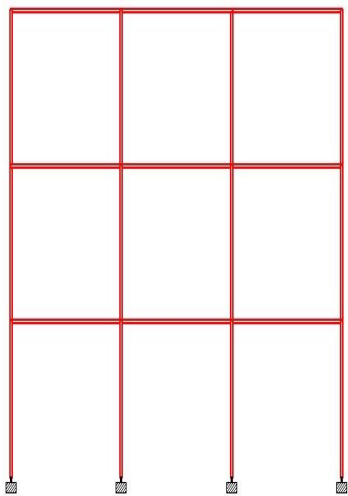


Figure 4: Sectional view of 3 storey model

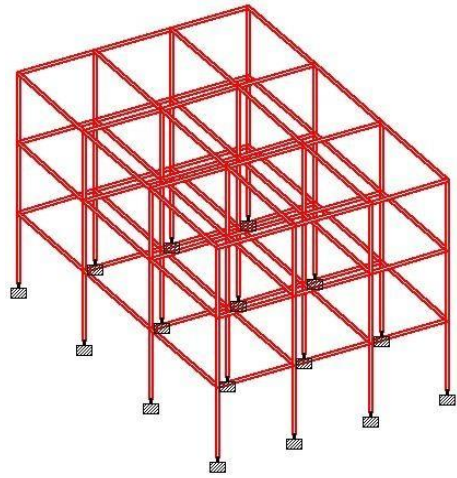


Figure 5: Isometric view of 3 storey structure model

4. Conclusion

Model analysis of 3 storey structure discloses that there is enormous difference in frequencies obtained but there is no enormous differences in dislocation in direction x and direction y respectively. Property of ductility of a moment opposing frame made from steel is to various degree exaggerated through its tallness. It means when base isolation technique is used the tallness dependency of property of ductility is highly putting consequence of great magnification. The combination of brace and steel dual system displays highly the property of ductility and hence the behavioral factor is also higher. By using the base isolation system, the flow of storey was minimized by 92% which shows how important and useful base isolation system is when steel framed structures are used. Pushover analysis is very helpful mechanism for admittance of strength elastically and also requisites for the deflections and also displaying weak areas of the structure. Succession of

configuration of artificial pivot inside the border constituent can exist as obviously observed in the ray merely. The structure undoubtedly acts like the physically powerful line and scrawny ray instrument.

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